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THE FOLLOWING NOTES APPLY, UNLESS OTHERWISE NOTED OR SHOWN ON PLANS.

CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS PRIOR TO STARTING WORK OR FABRICATION OF NEW STRUCTURAL MEMBERS. NOTIFY THE OWNER OF ANY CONFLICTS IN WRITING. DO NOT PROCEED WITH AFFECTED WORK UNTIL CONFLICTS HAVE BEEN RESOLVED.

WORK SHALL COMPLY WITH THE FOLLOWING: THE VIRGINIA UNIFORM STATEWIDE BUILDING CODE (VUSBC): 2021 EDITION THE INTERNATIONAL BUILDING CODE (IBC): 2021 EDITION AS AMENDED BY THE VUSBC ALL APPLICABLE STATE AND LOCAL CODES, ORDINANCES AND REGULATIONS.

- SECTIONS AND DETAILS SHOWN ON DRAWINGS ARE SITE SPECIFIC AND MAY NOT BE USED AT OTHER PROJECT LOCATIONS.
- 4. INSTALL MATERIALS PER MANUFACTURERS' RECOMMENDATIONS AND SPECIFICATIONS(SEE SHEET 3 OF 3 OF THIS PLAN)

# STRUCTURAL DESIGN NOTES (VCC 2021 & ASCE/SEI 7-22):

DESIGN CRITERIA:

STRUCTURAL DESIGN OF CONCRETE MASONRY RETAINING WALL SYSTEM

A. RISK CATEGORY = II (TABLE 1.5-1 ASCE/SEI 7-22/VCC TABLE 1604.5)

B. GROUND SNOW LOAD:Pg = 20 PSF, Pf = 14 PSF SNOW EXPOSURE FACTOR:Ce = 1.0 SNOW LOAD IMPORTANCE FACTOR I THERMAL FACTOR:Ct = 1.0

- C. ULTIMATE WIND SPEED = 115 MPH (EXPOSURE B)
- D. UNIFORM LIVE LOAD FOR YARDS/TERRACES = 100 PSF
- E. SOIL DENSITY = 120 PCF

SEISMIC DESIGN LOADS: (ASCE 7-22 CHAPTER 11 & ASCE HAZARD TOOL WEBSITE)

SEISMIC IMPORTANCE FACTOR = 1

MAPPED SPECTRAL RESPONSE ACCELERATION Ss = 0.16

MAPPED SPECTRAL RESPONSE ACCELERATION S1 = 0.044

SITE SOIL CLASS = D

SPECTRAL COEFFICIENT - Sds = 0.13

SPECTRAL COEFFICIENT - Sd1 = 0.063

SEISMIC DESIGN CATEGORY = B

#### **GRAVEL FOUNDATION NOTES**

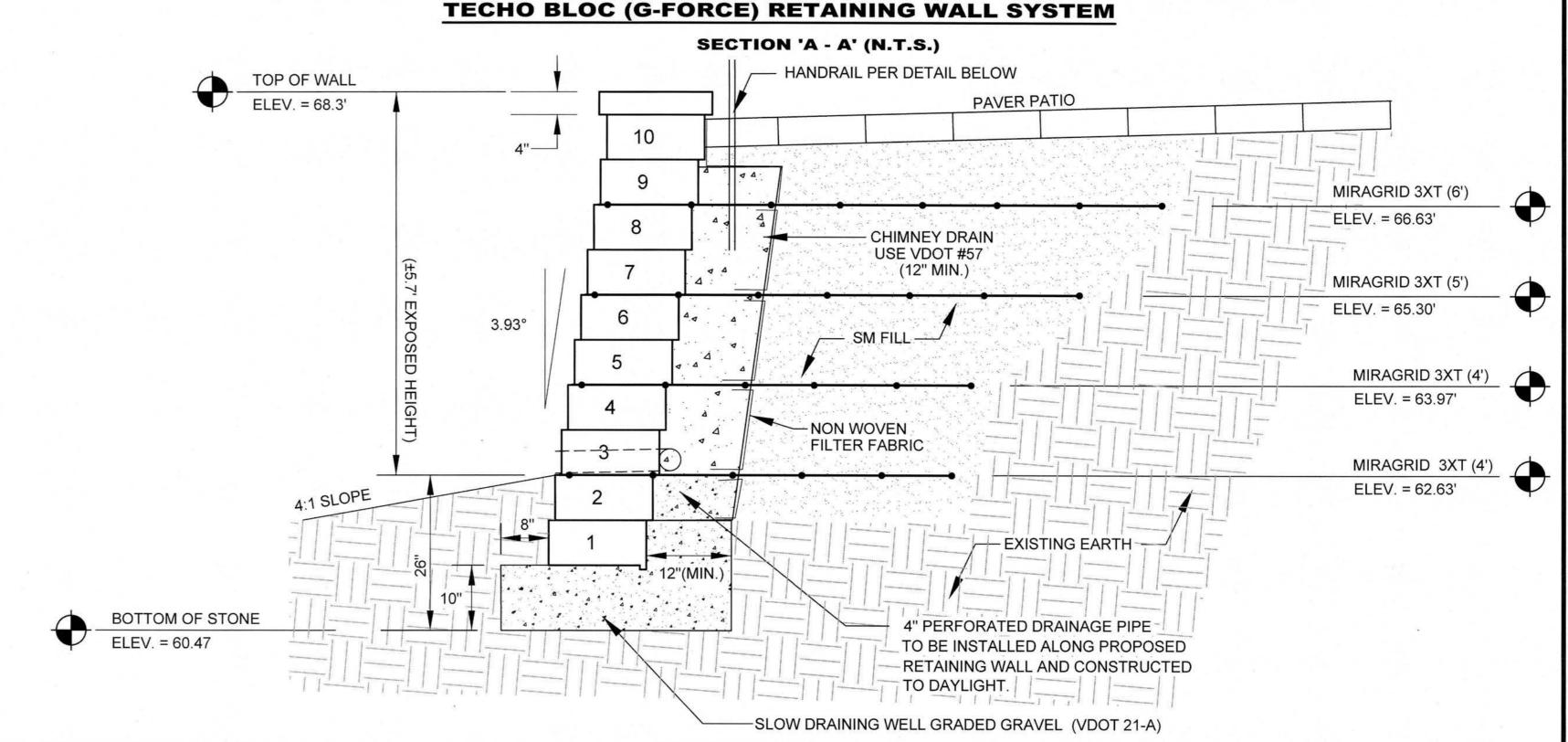
- 1. PLACE FOUNDATIONS ON FIRM, UNDISTURBED BEARING SOILS UNLESS FOUNDATIONS ARE PLACED ON COMPACTED FILL. (GEOTECHNICAL ENGINEER TO VERIFY 2,000 PSF BEARING
- 2. BACKFILL 12" MINIMUM BEHIND WALL AS SPECIFIED ON SHEET 2 OF 3 OF THIS PLAN.
- 3. COMPACTED BACKFILL: BACKFILL AND COMPACT TO NOT LESS THAN 95 PERCENT OF MAXIMUM DRY DENSITY ACCORDING TO ASTM D 1557. PLACE BACKFILL AND FILL MATERIALS IN LAYERS NOT MORE THAN 8 INCHES IN LOOSE DEPTH.
- 4. RETAINING WALL DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF.

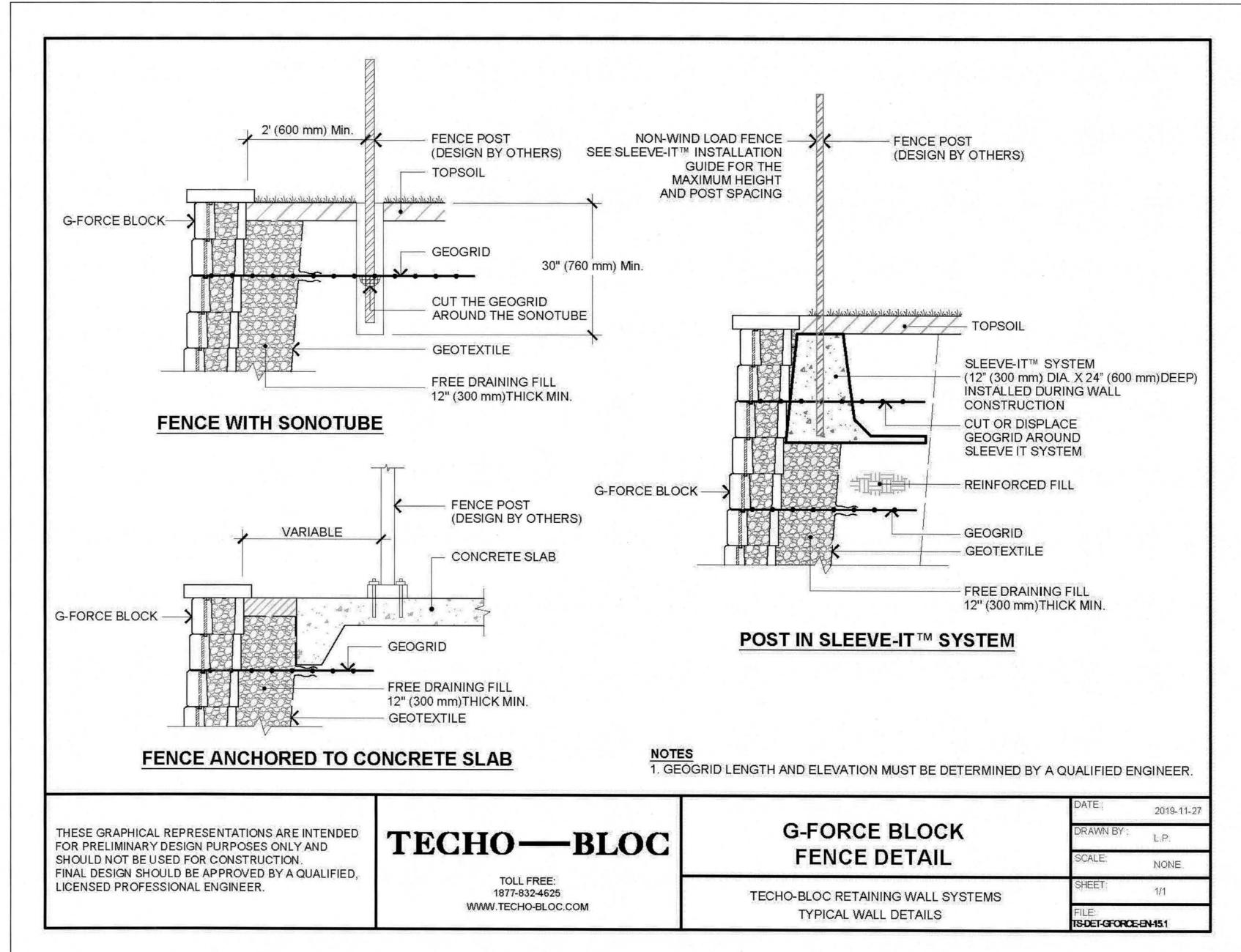
City of Williamsburg & County of James City Circuit Court: This PLAT was recorded on OCTOBER 1, 2025 at 1:44 AM/EM, PB — PG —

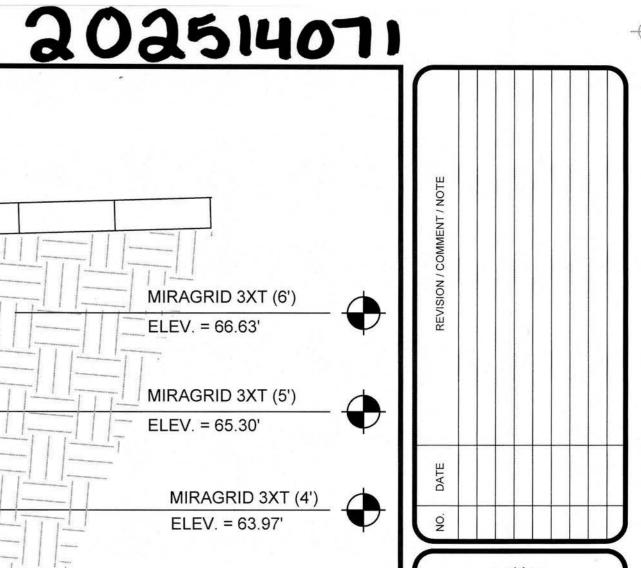
Document # 202514071

ELIZABETH E. O'CONNOR, CLERK Elizabeth E. O'Connor

Carge Small Plat(s) Recorded herewith as # 202514071











Date: 09/12/2025 Job Number: 25-307 Designed By: MHC Sheet Title: RETAINING WALL

PLAN

Sheet Number: 2 OF 3

TOTAL LBS.

PER ACRE

75 LBS.

175-200 LBS.

200-250 LBS.

40 LBS. (UNHULLED)

SITE SPECIFIC SEEDING MIXTURES

FOR COASTAL PLAIN AREA

#### MATERIALS SPECIFICATIONS

PER THE NATIONAL CONCRETE MASONRY ASSOCIATION'S DESIGN MANUAL FOR SEGMENTAL RETAINING WALLS, 3rd EDITION (2010)

## PART 1: GENERAL

#### 1.1 DESCRIPTION

ALL WORK SHALL CONSIST OF FURNISHING ALL MATERIALS, LABOR, EQUIPMENT, AND SUPERVISION TO INSTALL A SEGMENTAL RETAINING WALL SYSTEM IN ACCORDANCE WITH THESE SPECIFICATIONS AND IN REASONABLY CLOSE CONFORMITY WITH THE LINES, GRADES, DESIGN AND DIMENSIONS SHOWN ON THE PLANS OR AS ESTABLISHED BY THE OWNER OR OWNER'S ENGINEER.

#### 1.2 REFERENCE STANDARDS

#### 1.2.1 ENGINEERING DESIGN

- A. NCMA DESIGN MANUAL FOR SEGMENTAL RETAINING WALLS, 3rd EDITION (2010) B. ASTM D 6638 - STANDARD TEST METHOD FOR DETERMINING THE CONNECTION STRENGTH BETWEEN GEOSYNTHETICS REINFORCEMENT AND SEGMENTAL CONCRETE UNITS
- C. ASTM D 6916 STANDARD TEST METHOD FOR DETERMINING THE SHEAR STRENGTH BETWEEN SEGMENTAL
- 1.2.2 SEGMENTAL RETAINING WALL UNITS A. ASTM C 140 - STANDARD TEST METHODS FOR SAMPLING AND TESTING CONCRETE MASONRY UNITS AND RELATED UNITS 1262 - STANDARD TEST METHOD FOR EVALUATING THE FREEZE-THAW DURABILITY OF
- MANUFACTURED CONCRETE MASONRY UNITS AND RELATED CONCRETE UNITS C. ASTM C 1372 - STANDARD SPECIFICATION FOR DRY-CAST SEGMENTAL RETAINING WALL UNITS

### 1.2.3 GEOSYNTHETIC REINFORCEMENT A. ASTM D 4596 - STANDARD TEST METHOD FOR TENSILE PROPERTIES OF GEOTEXTILES BY THE WIDE-WIDTH

- B. ASTM D 5262 STANDARD TEST METHODS FOR EVALUATING THE UNCONFINED TENSION CREEP AND CREEP RUPTURE BEHAVIOR OF GEOSYNTHETICS C. ASTM D 5321 - STANDARD TEST METHOD FOR DETERMINING THE COEFFICIENT OF SOIL AND GEOSYNTHETIC OR GEOSYNTHETIC AND GEOSYNTHETIC FRICTION BY THE DIRECT SHEAR METHOD
- D. ASTM D 5818 STANDARD PRACTICE FOR EXPOSURE AND RETRIEVAL OF SAMPLES TO EVALUATE INSTALLATION DAMAGE OF GEOSYNTHETICS E. ASTM D 6637 - STANDARD TEST METHOD FOR DETERMINING TENSILE PROPERTIES OF GEOGRIDS BY THE
- SINGLE OR MULTI-RIB TENSILE METHOD ASTM D 6706 - STANDARD TEST METHOD FOR MEASURING GEOSYTHETIC PULLOUT RESISTANCE IN SOIL ASTM D 6992 - STANDARD TEST METHOD FOR ACCELERATED TENSILE CREEP AND CREEP-RUPTURE OF

# GEOSYNTHETIC MATERIALS BASED ON TIME-TEMPERATURE SUPERPOSITION USING STEPPED ISOTHERMAL

# 1.2.4 SOILS

- ASTM D 422 STANDARD TEST METHOD FOR PARTICLE-SIZE ANALYSIS OF SOILS
- B. ASTM D 698 STANDARDS TEST METHOD FOR LABORATORY COMPACTION CHARACTERISTICS OF SOIL USING STANDARD EFFORT (12 400 ft-lbf/ft3 (2,700 kN-m/m3)) C. ASTM D 1556 - STANDARD TEST METHOD FOR DENSITY AND UNIT WEIGHT OF SOIL IN PLACE BY THE SAND-CONE METHOD.
- D. ASTM D 1557 STANDARD TEST METHOD FOR LABORATORY COMPACTION CHARACTERISTICS OF SOIL USING MODIFIED EFFORT (56,000 ft-lbf/ft $^3$  (2,700 kN-m/m $^3$ )) E. ASTM D 2487 - STANDARD PRACTICE FOR CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES (UNIFIED SOIL CLASSIFICATION SYSTEM)
- F. ASTM D 6938 STANDARD TEST METHOD FOR IN-PLACE DENSITY AND WATER CONTENT OF SOIL AND SOIL-AGGREGATE BY NUCLEAR METHODS (SHALLOW DEPTH) G. ASTM D 4318 - STANDARD TEST METHODS FOR LIQUID LIMIT, PLASTIC LIMIT, AND PLASTICITY INDEX OF
- H. ASTM D 6913 STANDARD TEST METHODS FOR PARTICLE-SIZE DISTRIBUTION (GRADATION) OF SOILS USING
- SIEVE ANALYSIS ASTM G 51 - STANDARD TEST METHOD FOR MEASURING PH OF SOIL FOR USE IN CORROSION TESTING

# 1.2.5 DRAIN PIPE

- A. ASTM F 405 STANDARD SPECIFICATION FOR CORRUGATED POLYETHYLENE (PE) PIPE AND FITTINGS B. ASTM F 758 - STANDARD SPECIFICATION FOR SMOOTH-WALL POLY(VINYL CHLORIDE) (PVC) PLASTIC UNDERDRAIN SYSTEMS FOR HIGHWAY, AIRPORT, AND SIMILAR DRAINAGE
- 1.2.7 WHERE SPECIFICATIONS AND REFERENCE DOCUMENTS CONFLICT, THE OWNER'S ENGINEER SHALL MAKE THE FINAL DETERMINATION OF THE APPLICABLE DOCUMENT.

#### 1.3 APPROVED SEGMENTAL RETAINING WALL SYSTEMS

#### 1.3.1 SEGMENTAL WALL UNITS A. DIAMOND PRO 10D SYSTEM

1.3.2 GEOSYNTHETIC REINFORCEMENTS A. MIRAFI MIRAGRID 3XT

# 1.4 SUBMITTALS

# 1.4.1 MATERIAL SUBMITTALS

- A. THE CONTRACTOR SHALL SUBMIT MANUFACTURER'S CERTIFICATIONS, 30 DAYS PRIOR TO THE START OF WORK, STATING THAT THE SRW UNITS, GEOSYNTHETIC REINFORCEMENT, REINFORCED BACKFILL, AND GRAVEL FILL MEET THE REQUIREMENTS OF PART 2 OF THIS SPECIFICATION. THE CONTRACTOR SHALL PROVIDE A LIST OF SUCCESSFUL PROJECTS WITH REFERENCES SHOWING THAT THE INSTALLER FOR THE SEGMENTAL RETAINING WALL IS QUALIFIED AND HAS A RECORD OF SUCCESSFUL PERFORMANCE.
- 1.5 DELIVERY, STORAGE & HANDLING THE CONTRACTOR SHALL INSPECT THE MATERIALS UPON DELIVERY TO ASSURE THAT PROPER TYPE AND GRADE OF MATERIAL HAS BEEN RECEIVED. B. THE CONTRACTOR SHALL STORE AND HANDLE MATERIALS IN ACCORDANCE WITH MANUFACTURER'S
- RECOMMENDATIONS AND IN A MANNER TO PREVENT DETERIORATION OR DAMAGE DUE TO MOISTURE. TEMPERATURE CHANGES, CONTAMINANTS, CORROSION, BREAKING, CHIPPING OR OTHER CAUSES. C. THE CONTRACTOR SHALL PROTECT THE MATERIALS FROM DAMAGE. DAMAGED MATERIAL SHALL NOT BE INCORPORATED INTO RETAINING WALLS SHOWN WITHIN THESE PLANS.

# PART 2: MATERIAL

- 2.1 CONCRETE SEGMENTAL RETAINING WALL UNITS
- CONCRETE SEGMENTAL UNITS SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 1372 AND HAVE A MINIMUM NET AVERAGE 28 DAYS COMPRESSIVE STRENGTH OF 3,000 PSI AND A MAXIMUM ABSORPTION OF 13 pcf (FOR NORMAL WEIGHT) AS DETERMINED IN ACCORDANCE WITH ASTM C 140. FOR AREAS SUBJECT TO DETRIMENTAL FREEZE-THAW CYCLES, AS DETERMINED BY THE OWNER OR OWNER'S ENGINEER, THE CONCRETE SHALL HAVE ADEQUATE FREEZE/THAW PROTECTION NAND MEET THE REQUIREMENTS OF ASTM C 1372 WHEN TESTED IN ACCORDANCE WITH ASTM C 1262.
- ALL UNITS SHALL BE SOUND AND FREE OF CRACKS OR OTHER DEFECTS THAT WOULD INTERFERE WITH THE PROPER PLACING OF THE UNIT OR SIGNIFICANTLY IMPAIR THE STRENGTH OR PERMANENCE OF THE CONSTRUCTION. ANY CRACKS OR CHIPS OBSERVED DURING CONSTRUCTION SHALL FALL WITHIN THE GUIDELINES OUTLINED IN ASTM C 1372.
- SRW UNITS DIMENSIONS SHALL NOT DIFFER MORE THAN ± 1/8 in., AS MEASURED IN ACCORDANCE WITH ASTM C 140. THIS TOLERANCE DOES NOT APPLY TO ARCHITECTURAL SURFACES, SUCH AS SPLIT FACES.
- SRW UNITS SHALL MATCH THE COLOR, SURFACE FINISH, AND DIMENSION FOR HEIGHT, WIDTH, DEPTH, AND BATTER AS SHOWN ON THE PLANS.
- IF PINS OR CLIPS ARE USED BY THE RETAINING WALL SUPPLIER TO INTERCONNECT SRW UNITS, THEY SHALL CONSIST OF A NONDEGRADING POLYMER OR GALVANIZED STEEL AND BE MADE FOR THE EXPRESS USE WITH
- THE SRW UNITS SUPPLIED. CAP ADHESIVE SHALL MEET THE REQUIREMENTS OF THE SRW UNIT MANUFACTURER.

- 2.2 GEOSYNTHETIC REINFORCEMENTS GEOSYNTHETIC REINFORCEMENTS SHALL CONSIST OF HIGH TENACITY PET GEOGRIDS, HDPE GEOGRIDS, OR GEOTEXTILES MANUFACTURED FOR SOIL REINFORCEMENT APPLICATIONS. THE TYPE, STRENGTH AND PLACEMENT LOCATION OF THE REINFORCING GEOSYNTHETIC SHALL BE AS SHOWN ON THE PLANS. THE DESIGN PROPERTIES OF THE REINFORCEMENT SHALL BE DETERMINED ACCORDING TO THE PROCEDURES OUTLINED IN THIS SPECIFICATION AND THE NCMA DESIGN MANUAL FOR SEGMENTAL RETAINING WALLS, 3rd EDITION (2009). DETAILED TEST DATA SHALL BE SUBMITTED TO THE OWNER'S ENGINEER FOR APPROVAL AT LEAST 30 DAYS PRIOR TO CONSTRUCTION AND SHALL INCLUDE TENSILE STRENGTH (ASTM D 4595 OR ASTM D 6637), CREEP (ASTM D 5262), SITE DAMAGE (ASTM D 5818), DURABILITY (FHWA GUIDANCE (FHWA
- THE DRAINAGE COLLECTION PIPE SHALL BE A PERFORATED OR SLOTTED PVC OR CORRUGATED HDPE PIPE. THE PIPE AND GRAVEL FILL MAY BE WRAPPED WITH A GEOTEXTILE THAT WILL FUNCTION AS A FILTER. DRAINAGE PIPE SHALL BE MANUFACTURED IN ACCORDANCE WITH ASTM F 405 OR ASTM F 758.
- GRAVEL FILL SHALL BE A VDOT #57 STONE OR CLEAN CRUSHED STONE OR GRANULAR FILL MEETING THE FOLLOWING GRADATION AS DETERMINED IN ACCORDANCE WITH ASTM D 422:

NGI-00-043, FHWA NHI-00-044)), PULLOUT (ASTM D 6706), DIRECT SHEAR (ASTM D 5321 AND) AND

#### PERCENT PASSING 3/4 in. 75-100 0-60 No. 4

CONNECTION (ASTM D 6638) TEST DATA.

No. 200

REINFORCED BACKFILL THE REINFORCED BACKFILL SHALL BE FREE OF DEBRIS AND CONSIST OF ONE OF THE FOLLOWING INORGANIC USCS SOIL TYPES: GP, GW, SW, SP, SM, MEETING THE FOLLOWING GRADATION AS DETERMINED IN ACCORDANCE WITH ASTM D 422.

0-5

SIEVE SIZE	PERCENT PASSING
1 in.	100
3/4 in.	100-20
No. 4	0-60
No. 200	$0-35^{2.5(B)}$

ASTM D 4833

- B. COHESIONLESS, COARSE-GRAINED SOILS ARE PREFERRED; FINER SOILS WITH LOW-PLASTICITY (PI OF FINES<20) MAY BE USED ONLY WITH THE CONSENT OF THE OWNER'S ENGINEER FIRST. WITH THE CONSENT,
- THE ENGINEER SHALL GIVE ADDITIONAL DESIGN CRITERIA TO BE IMPLEMENTED. THE pH OF THE BACKFILL MATERIAL SHALL BE BETWEEN 3 AND 9 WHEN TESTED IN ACCORDANCE WITH
- DRAINAGE GEOTEXTILE SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES OR SHALL MEET THE CRITERIA RECOMMENDED BY THE WALL DESIGN ENGINEER: GRAB TENSILE ASTM D 4632 TRAP TEAR ASTM D 4533 WATER FLOW RATE ASTM D 4491

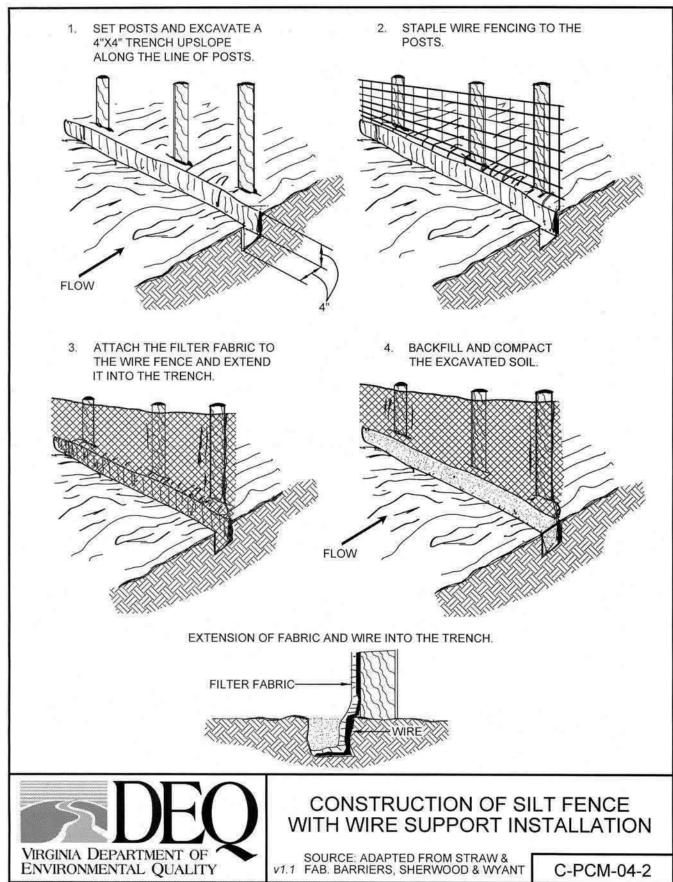
#### PART 3: CONSTRUCTION

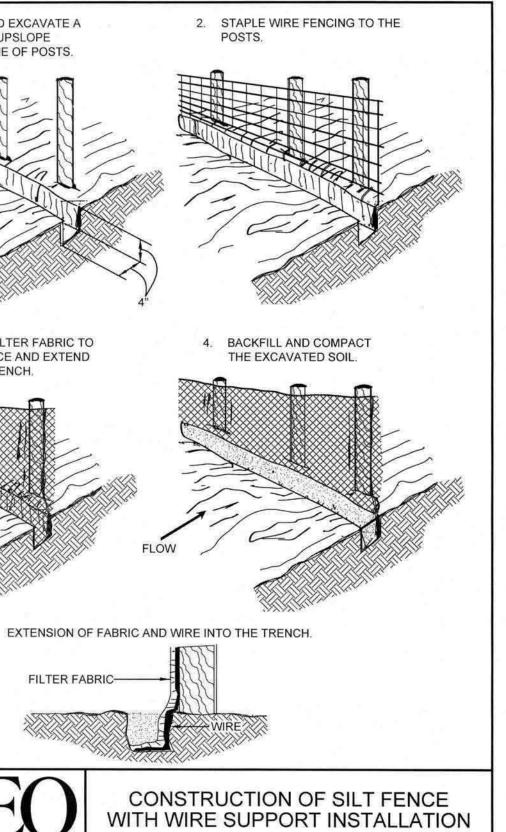
**PUNCTURE** 

- 3.1 CONSTRUCTION OBSERVATION
- A. THE OWNER OR OWNER'S ENGINEER SHOULD VERIFY THE MATERIALS SUPPLIED BY THE CONTRACTOR MEET ALL THE REQUIREMENTS OF THE SPECIFICATION. THIS INCLUDES ALL SUBMITTALS AND PROPER INSTALLATION OF THE SYSTEM.
- B. THE CONTRACTOR'S FIELD CONSTRUCTION SUPERVISOR SHALL HAVE DEMONSTRATED EXPERIENCE AND BE QUALIFIED TO DIRECT ALL WORK AT THE SITE.

PLACEMENT.

- A. THE CONTRACTOR SHALL EXCAVATE TO THE LINES AND GRADES SHOWN ON THE PLANS. THE CONTRACTOR SHALL TAKE PRECAUTIONS TO MINIMIZE OVER-EXCAVATION. EXCAVATION SUPPORT, IF REQUIRED, SHALL BE DESIGNED BY THE CONTRACTOR. 3.3 FOUNDATION PREPARATION
- A. FOLLOWING EXCAVATION FOR THE LEVELING PAD AND THE REINFORCED SOIL ZONE, FOUNDATION SOIL SHALL BE EXAMINED BY THE OWNER'S GEOTECHNICAL ENGINEER TO ASSURE THE ACTUAL FOUNDATION SOIL STRENGTH MEETS OR EXCEEDS THE ASSUMED DESIGN BEARING STRENGTH. SOILS NOT MEETING THE REQUIRED STRENGTH SHALL BE REMOVED AND REPLACED WITH SOIL MEETING THE DESIGN CRITERIA, AS DIRECTED BY THE OWNER'S GEOTECHNICAL ENGINEER.
- 3.4 LEVELING PAD PREPARATION A MINIMUM 6 in. THICK LAYER OF COMPACTED GRANULAR MATERIAL SHALL BE PLACED FOR USE AS A LEVELING PAD UP TO THE GRADES AND LOCATIONS AS SHOWN ON THE CONSTRUCTION DRAWINGS. THE GRANULAR BASE SHALL BE COMPACTED TO PROVIDE A FIRM, LEVEL BEARING PAD ON WHICH TO PLACE THE FIRST COURSE OF CONCRETE SRW UNITS. A LEVELING PAD CONSISTING OF 6 in. (MINIMUM) THICK LEAN, UNREINFORCED CONCRETE MAY BE USED AT THE WALL CONTRACTOR'S OPTION, OR IF SO DETAILED ON THE PLANS. THE LEVELING PAD SHOULD EXTEND A MINIMUM OF 6 in. FROM THE TOE AND HEEL OF THE SRW
- 3.5 SRW AND GEOSYNTHETIC REINFORCEMENT PLACEMENT A. ALL MATERIALS SHALL BE INSTALLED AT THE PROPER ELEVATION NAND ORIENTATION AS SHOWN IN THE WALL DETAILS ON THE CONSTRUCTION PLANS OR AS DIRECTED BY THE OWNER'S ENGINEER. THE CONCRETE SEGMENTAL WALL UNITS AND GEOGYNTHETIC REINFORCEMENT SHALL BE INSTALLED IN GENERAL ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. THE DRAWINGS SHALL GOVERN IN ANY CONFLICT BETWEEN THE TWO REQUIREMENTS.
- B. OVERLAP OR SPLICE CONNECTIONS OF THE GEOSYNTHETIC IN THE DESIGN STRENGTH DIRECTION SHALL NOT BE PERMITTED. THE DESIGN STRENGTH DIRECTION IS THAT LENGTH OF GEOSYNTHETIC REINFORCEMENT PERPENDICULAR TO THE WALL FACE AND SHALL CONSIST OF ONE CONTINUOUS PIECE OF MATERIAL. ADJACENT SECTIONS OF GEOSYNTHETIC SHALL BE PLACED IN A MANNER TO ASSURE THAT THE HORIZONTAL COVERAGE SHOWN ON THE PLANS IS PROVIDED. . GEOSYNTHETIC REINFORCEMENT SHOULD BE INSTALLED UNDER TENSION. A NOMINAL TENSION SHALL BE
- APPLIED TO THE REINFORCEMENT AND MAINTAINED BY STAPLES, STAKES, OR HAND TENSIONING UNTIL THE REINFORCEMENT HAS BEEN COVERED BY AT LEAST 6 in. OF SOIL FILL. BROKEN, CHIPPED, STAINED OR OTHERWISE DAMAGED UNITS SHALL NOT BE PLACED IN THE WALL UNLESS
- THEY ARE REPAIRED, AND THE REPAIR METHOD AND RESULTS ARE APPROVED BY THE SRW DESIGN 3.6 BACKFILL PLACEMENT A. THE REINFORCED BACKFILL SHALL BE PLACED AS SHOWN IN CONSTRUCTION PLANS IN MAXIMUM COMPACTED LIFT THICKNESS OF 8 in. AND SHALL BE COMPACTED TO A MINIMUM 95% OF STANDARD PROCTOR DENSITY
- BACKFILL SHALL BE PLACED SPREAD AND COMPACTED IN SUCH A MANNER THAT MINIMIZED THE DEVELOPMENT OF WRINKLES OR MOVEMENT OF THE GEOSYNTHETIC REINFORCEMENT AND THE WALL FACING B. ONLY HAND-OPERATED COMPACTION EQUIPMENT SHALL BE ALLOWED WITHIN 3 ft OF THE FRONT OF THE WALL FACE. A MAXIMUM COMPACTED LIFT THICKNESS OF 8 in. SHALL BE USED IN THIS ZONE. SOIL DENSITY IN THIS AREA SHALL NOT BE LESS THAN 95% PROCTOR DENSITY WITHOUT AFFECTING WALL ALIGNMENT. SOIL
- DENSITY TESTING IN THIS AREA SHOULD BE VERIFIED BY FIELD DENSITY TESTING. CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE GEOSYNTHETIC REINFORCEMENT. A MINIMUM BACKFILL THICKNESS OF 6 in. IS REQUIRED PRIOR TO OPERATION OF TRACKED VEHICLES OVER THE GEOSYNTHETIC REINFORCEMENT. TURNING OF TRACKED VEHICLES SHOULD BE KEPT TO A MINIMUM TO
- PREVENT DISPLACING THE FILL AND DAMAGING OR MOVING THE GEOSYNTHETIC REINFORCEMENT. AT THE END OF EACH DAY'S OPERATION, THE WALL CONTRACTOR SHALL SLOPE THE LAST LEVEL OF BACKFILL AWAY FROM THE WALL FACING TO DIRECT RUNOFF OF RAINWATER AWAY FROM THE WALL FACE. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING SURFACE RUNOFF FROM ADJACENT AREAS IS NOT ALLOWED TO ENTER THE WALL CONSTRUCTION AREA.
- 3.7 GRAVEL FILL AND DRAINAGE PLACEMENT A. GRAVEL FILL SHALL BE PLACED TO THE MINIMUM FINISHED THICKNESS AND WIDTHS SHOWN ON THE
- CONSTRUCTION PLANS. DRAINAGE COLLECTION PIPES SHALL BE INSTALLED TO MAINTAIN GRAVITY FLOW OF WATER OUTSIDE OF THE REINFORCED SOIL ZONE. THE DRAINAGE COLLECTION PIPE SHOULD DAYLIGHT INTO A STORM SEWER MANHOLE OR ALONG A SLOPE AT AN ELEVATION LOWER THAN THE LOWEST POINT OF THE PIPE WITHIN THE AGGREGATE
- C. THE MAIN COLLECTION DRAIN PIPE, JUST BEHIND THE BLOCK FACING, SHALL BE A MINIMUM OF 3 in. IN DIAMETER.E THE SECONDARY COLLECTION DRAIN PIPES SHOULD BE SLOPED A MINIMUM OF TWO PERCENT TO PROVIDE GRAVITY FLOW INTO THE MAIN COLLECTION DRAIN PIPE. DRAINAGE LATERALS SHALL BE SPACED AT A MAXIMUM 50 ft. SPACING ALONG THE WALL FACE.
- 3.8 CAP BLOCK PLACEMENT THE CAP BLOCK AND/OR TOP SRW UNIT SHALL BE BONDED TO THE SRW UNITS BELOW USING CAP ADHESIVE DESCRIBED IN PART 2.01F. THE BLOCK SHALL BE DRY AND SWEPT CLEAN PRIOR TO ADHESIVE





30 LBS. (HULLED) -HYBRID BERMUDAGRASS (BY OTHER VEGETATIVE ESTABLISHMENT METHOD, SEE STD. & SPEC. 3.34) GENERAL SLOPE (3:1 OR LESS) -KENTUCKY 31 FESCUE 128 LBS. -RED TOP GRASS 2 LBS. 20 LBS. 150 LBS. -SEASONAL NURSE CROP \* LOW MAINTENANCE SLOPE (STEEPER THAN 3:1) -Kentucky 31 tall fescue 93-108 LBS. -COMMON BERMUDAGRASS \*\* 0-15 LBS. -RED TOP GRASS 2 LBS. -SEASONAL NURSE CROP \* 20 LBS. -SERICEA LESPEDEZA \*\* 150 LBS. \* USE SEASONAL CROP IN ACCORDANCE WITH SEEDING DATES AS STATED BELOW: FEBRUARY, MARCH THROUGH APRIL....

MINIMUM CARE LAWN

COMMERCIAL OR RESIDENTIAL

HIGH-MAINTENANCE LAWN

-COMMON BERMUDA GRASS \*\*

-HYBRID BERMUDAGRASS (SEED)\*\*

-KENTUCKY 31 OR TURF-TYPE TALL FESCUE

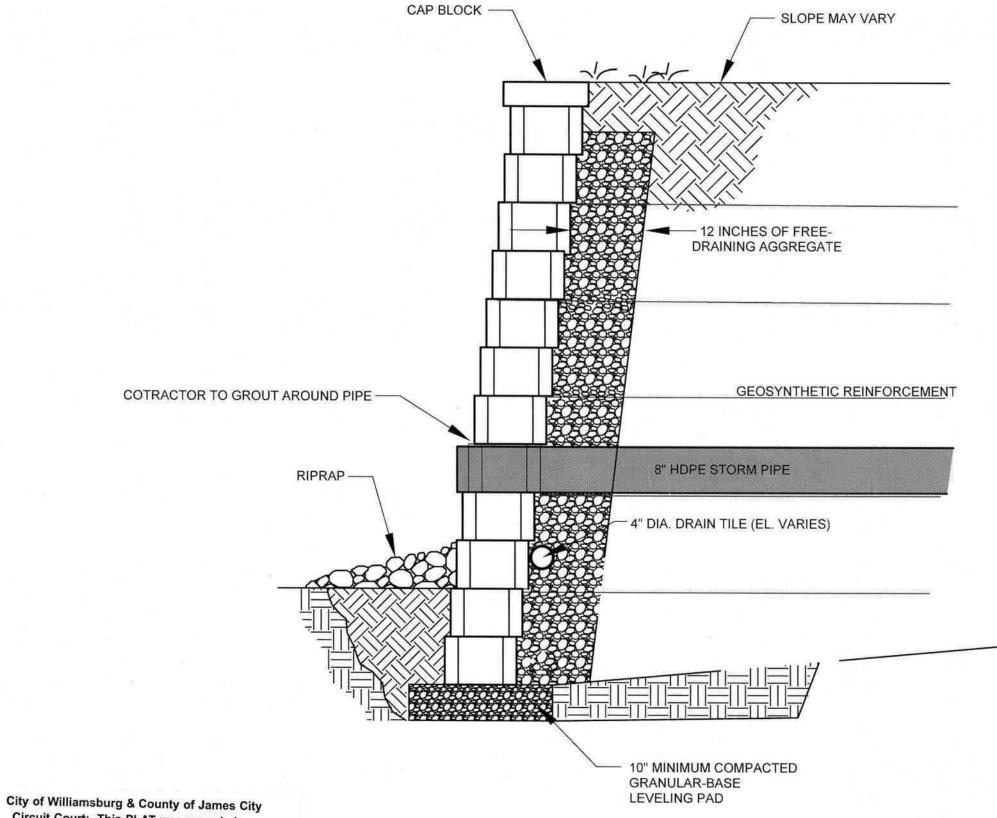
-KENTUCKY 31 OR TURF-TYPE TALL FESCUE

.ANNUAL RYE MAY 1ST THROUGH AUGUST... FOXTAIL MILLET SEPTEMBER. OCTOBER THROUGH NOVEMBER 15TH.... ANNUAL RYE NOVEMBER 16TH THROUGH JANUARY..... .. WINTER RYE

\*\* MAY THROUGH OCTOBER, USE HULLED SEED. ALL OTHER SEEDING PERIODS, USE UNHULLED SEED. WEEPING LOVEGRASS MAY BE ADDED TO ANY SLOPE OR LOW-MAINTENANCE MIX DURING WARMER SEEDING PERIODS; ADD 10-20 LBS./ACRE IN

\*\*\* NUTRIENTS SHALL BE APPLIED IN ACCORDANCE WITH MANUFACTER'S RECOMMENDATIONS AND SHALL NOT BE APPLIED DURING RAINFALL EVENTS.

Table 3.32-D



Circuit Court: This PLAT was recorded on at\_1:44\_AM(PM) PB\_\_\_\_ PG\_\_\_ Document # 202514071 **ELIZABETH E. O'CONNOR, CLERK** 

Elizabeth E. O' Connor

SECTION VIEW (N.T.S.)





03

Scale: N/A Date: 09/12/2025 Job Number: 25-307 Designed By: MHC

Sheet Title:

RETAINING WALL PLAN

Sheet Number 3 OF 3